

Bearing Resistance Failure GBR+

Inhaltsverzeichnis

3 4 6
4 6 6
6
6
•
7
7
7
8
11
11
12
13
15
15
16
17
17
19
20
21
22

Basic Documentation – Overview

In addition to the individual program manuals, you will find basic explanations on the operation of the programs on our homepage <u>www.frilo.com</u> in the Campus-download-section.



Application options

The GBR+ application is suitable for the verification of squared and rectangular foundations. External loads can optionally apply centrically or with a uniaxial or biaxial eccentricity.

The software application calculates the soil pressure underneath the four corner points and the position of the zero-line in combination with a gaping joint.

The gaping joint, the permissible bearing pressure, the sliding resistance and the position stability as well as of the resistance to ground failure are verified for the foundation.

The structural system consists of the foundation slab and optionally includes a cast-on column (or plinth) with or without eccentricity.

The user can include the following load types in the calculation:

- Single vertical load V at the column location
- Horizontal loads Hx and Hy optionally at the top edge of the column or in the foundation base
- Outer moments Mx and My
- Earth top load and an additional uniformly distributed load applying to the foundation surface without a column and additional vertical single loads applying at freely selectable points.

Standards

- DIN EN 1992
- ÖNORM EN 1992
- BS EN 1992
- NF EN 1992
- PN EN 1992
- EN 1992
- still available:
- DIN 1045-1
- ÖNORM B 4700

According to the selected reinforced concrete standard, the program automatically sets the associated foundation and ground failure standard.

- DIN EN 1997-1
- ÖNORM EN 1997-1
- BS EN 1997-1
- NF EN 1997-1
- PN EN 1997-1
- DIN 1054:1976/2005/2021

Support of all 3 verification methods according to Eurocode 7, adjustable for all national annexes.

The partial safety factors and combination equations for the geotechnical verifications can be edited. See Design - <u>Parameters</u>.

Properties	ф
Basic parameter	9.0
⊕ System	
⊕. Design	
- Output	





Additional option FDPro

With the additional option FDPro, the foundation programmes FD+/FDB+/FDS+ and GBR+ can be extended to include

- an earth pressure approach
- an inclined foundation base
- a seismic ground failure verification
- a ground failure punching shear verification
- a bearing capacity calculation of the foundation soil with a table of design values of the base pressure resistance.
- a graphical output of the internal forces along the main axes

See <u>calculation basis for foundation engineering</u> in the FD+ manual.

See also ▶ <u>Video</u>



Basis of calculation

Position stability

For the verification of the position stability, the stabilising and destabilising moments in relation to the outer edges of the foundation are calculated in combination with the Eurocodes. If you use the result load cases instead of the characteristic ones, they are used in the calculation of the stabilising and destabilising moments without consideration of reduction factors. In this case, only self-weight is multiplied with the partial safety factors that have a favourable or unfavourable effect.

Gaping joint

Under permanent loading, no gaping joint must occur and under the total loading, gaping of the foundation joint is allowed up to the centre of gravity. In combination with Eurocodes, the calculation of the gaping joint is based on representative loads instead of characteristic ones.

If you use result load cases instead of superpositions (Basic parameters > Type of loading), the loads are reduced to the characteristic level with the help reduction factors before examining the gaping joint. In this connection, it is important to define whether the individual load cases are the result of permanent loads exclusively or of both, permanent and variable loads: a gaping joint up to the centre of gravity is only permitted for the combination of permanent and variable loads. It is not permitted if only permanent loads apply.

Permissible bearing pressure

For a simplified verification in standard cases, the existing bearing pressure is compared to the permissible one. The permissible bearing pressure can be determined automatically with the help of standardised tables for the simplified verification. You can increase or reduce the permissible bearing pressure taken from the standard tables if the corresponding border conditions, such as the required anchoring depth, ground water or the relation of the horizontal and vertical loads, require this. In combination with Eurocodes, the calculation of the equivalent area for the design value of the bearing pressure is based on representative loads instead of characteristic ones.

If you use result load cases instead of superpositions, the loads are reduced to the characteristic level with the help reduction factors before examining the equivalent area. The design value of the bearing pressure is obtained by dividing the design value of the vertical loads by the representative or characteristic equivalent area. As additional information, the software determines the inclination of the characteristic or representative bearing pressure resultant in order to check whether the inclination is suitable for a simplified verification.

Stability against sliding

If horizontal forces apply, the stability against sliding is verified. It is considered satisfactory if $T_d \le R_{td}$.

 T_d : design value of the loads applying in parallel to the bottom of the foundation.

 T_d is calculated by the software by multiplying T_k with the partial safety factors for the decisive limit state. The software uses the partial safety factors for the permanent and quasi-permanent design situations. If you define loads by accidental actions or earthquakes, the accidental and earthquake design situations are taken into account as well.

Rtd:design value of the sliding resistance.Rtd is calculated by dividing Rtk by the partial safety factor for the sliding resistance for the decisivelimit state in accordance with the currently selected foundation standard.



Safety against ground failure

In combination with Eurocodes, the ground failure safety is calculated with characteristic or representative values. The design values of the ground failure resistance are determined by dividing the characteristic values by the partial safety coefficients. They are compared to the design values of the actions, which are multiplied by partial safety factors. Depending on the selected design standard, the characteristic or representative ground failure safety is calculated on the basis of ÖNORM B 4435-2 or DIN 4017.

The FD+, FDB+, FDS+ and FDR+ applications always calculate the ground failure safety as an isolated foundation. FDS+ and FDR+ calculate the ground failure safety as a strip foundation if the wall length corresponds to the foundation length.

In the GBR+ application, the "strip foundation" verification type is optionally available. When you select this type of verification, all shape coefficients and the load inclination coefficients 'ma' and 'mb' are set to 1.0. Instead of the arithmetical equivalent width in the longitudinal wall direction (y-direction) the foundation length (y-direction) is taken into account.



Definition of the structural system

The definition of properties and control parameters is done in the menu on the left side of the screen. You can check the effect of the entered values in the graphical representation on the right screen section. Before the first entry, you can change the units of measurement (cm, m ...) via File > <u>Settings</u> if required.

Wizard

The Wizard appears by default/automatically when the program starts, but can be switched off.

Input options in the 3D graphics

The description of the input options in the graphic window is given in the Document "Basic operating instructions PLUS".

Basic parameters

Type of actions

Design values	e loads shall be defined with their partial safety ctors. Under particular conditions, these values are duced by reduction factors for the foundation nalyses.						
Characteristic	the loads are specified with the characteristic (1.0-fold) value.						
Soil bearing resistand	ce If the option is selected, only the bearing capacity of the soil is output in the form of a table with the design values for the bearing pressure resistance.						

Standard reinforced concrete

Selection of the desired reinforced concrete standard.

The corresponding <u>foundation standard</u> is displayed in the graphics window at the top left





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System

System

Foundation Type

Here you select the desired foundation shape

- Rectangular foundation
- Strip foundation

which influences the shape coefficients in the bearing failure verification. See also **Basis of calculation**.

Location foundation

The global position related to the foundation axis is only required for communication with other programs such as GEO and SBR+.

Remarks

Click the button is to enter your own <u>comments</u> about the system.

Foundation

In the foundation ground plan, the x-axis (positive) runs from the left to the right and the y-axis (positive) from the bottom to the top.

Width x foundation dimension in x-direction

Length y foundation dimension in y-direction Height z foundation height Average anchoring depth d Lowest foundation depth below the ground level or the top edge of the basement floor.

Density y gamma concrete

Base inclination and a 4-sided different surface definition are possible with the additional option FL+ PRO.

Column

Width x	width of the column
Thickness y	thickness of the column
Height z	height of the column

Eccentricity

Eccentricity x	Column eccentricity in x-direction.
Eccentricity y	Column eccentricity in y-direction.

Properties			
Basic parameter System Foundation Column Ground wa Soil Ground wa Surface Loading Design Output	er n ster	3	۹ 🕲
System			0
Foundation	Тур	Rectangle foundation	-
Location foundat	tion	Rectangle foundation Strip Foundation	
x	х	[m]	0.00
У	У	[m]	0.00
z	z	[m]	0.00
Rotation angle	α	[°]	0.00
Remarks			0



Soil

Soil properties

Son properties			Properties			4		
Determination σR,d	Select whether th resistance should from a standard defined <u>(own) tab</u>	e design value of the bearing d be entered <u>directly</u> , or to come table (<u>DIN 1054</u>) or from a user <u>ble</u> - see section below.	Basic parameter System Soundation Column Soil	Q (2)				
Load-bearing resistand	ce Specification of t σR,d	he permissible bearing pressure	Ground water Surface					
Permissible settlemen	nt Permissible settle calculated settle	ement for comparison with the ment and presentation of the settlement verification	⊕. Design ⊕. Output					
Eff. friction Angle of		r friction underneath the foundation	Soil properties			0		
Eπ. metion Angle φ	base.	i metion underneath the roundation	Determination	oK,d	DIN 1054:2	021 -		
Soil friction angle	The soil friction a	angle is relevant for the sliding	cross section resistance	e adm	DIN 1054:2	021		
Soli metion angle	safety check If the	he angle of friction δ is not	Effective friction angle	5,00m.	From own ta	ble 32.5		
	determined sepa	rately, the characteristic angle of	Soil friction angle	δk	3/3 m	J2.J		
	friction ϕ 'k may b	be used instead of the critical angle	Soil friction angle	δk	- 1"]	32.5		
	of friction for in-s	situ concrete foundations. A value	Dialog	-	OD	en		
	of 35° must not b	e exceeded. The same applies to	First soil laver			0		
	prefabricated fou	indations if the precast elements	Stroke weight	v	[kN/m ²]	17.00		
	are laid in the mo	ortar bed. If the prefabricated	Buovant unit weight	Y V	[kN/m ²]	10.00		
	foundations are s	smooth and without a mortar bed,	Effective friction angle	n'	[4]	32.5		
	the characteristic	c soil friction angle $\delta k = 2/3 \phi' k$	Cohesion	¢	[kN/m²]	0.00		
	shall be used.		Dialog		OD	en		
	pressure resistar Click the "open"/	ace is taken from a table (standard o "edit" Button to open the tabledialog	r user defined)	sanny				
E	Parameters by stanc	dard table DIN 1054:	Bearing pressure resistance					
ļ	According to Annex	The soil pressure is taken from	Soil properties			0		
		the corresponding table in the	According to Appex	Tabl	AC 9	w v		
		Soli engineering standard or its	Consistence	riaid	e //0.0			
	.		Increase (geometry)	[%]		20.0		
(Consistence	consistency of soil: rigid, half-	Increase (strength)	[%]		50.0		
			Anchoring depth	d [m]		1.00		
1	ncrease (geometry) ncrease (strength)	The permissible bearing pressure c relevant border conditions (b/d) spe standard are satisfied. By ticking the edited. The permissible bearing pressure is sufficiently solid. By ticking this opt <i>Note: The values are added up under The subsoil has sufficient strength of foundation base that corresponds to foundation, but at least down to a definition.</i>	can be increased by 20 % if the pecified by the applicable this option the value can be is increased by 50 %, if the soil is ption the value can be edited. <i>The particular conditions (70 %)</i> . <i>The down to a depth below the</i> <i>to twice the width of the</i>					
		checks whether an increase in the d pressure resistance is permissible a	lesign value of the bea and then applies this.	ring				



Anchoring depth d	Lowest foundation depth below the ground level or the top edge of the basement floor.
From own table	
Create:	Generates a table with design values of the bearing pressure resistance from several parameters.
Edit:	Open the dialog to enter the design value of the bearing pressure resistance σ Rd. The value σ Rd should come from a geotechnical report and should have sufficient guarantees against ground failure and a sufficient limitation of settlements. Furthermore, the corresponding foundation width and anchoring depth must be specified.

The meaning of the other buttons can be seen from the Tooltips.

First soil layer

In this section you can enter the values of the first soil layer.

For additional soil layers click the Button "Dialog - open".

Buoyant unit weight	γ́	Specific weight of the soil layer under buoyancy. This value is only used if
		groundwater was defined (► System ► Soil)
Friction angle	φ́	Friction angle of the soil in this layer.

Cohesion c' Soil cohesion.

Further soil layers / additional values (> Dialog "open")

Library	Cat.	Name	Icon	Y	Y'	φ'	c'	xU'	v	Em	PI	α	qc	E'	Procedure	E*	Es	x	ks	both sides drained	Cα'
				[kN/m³]	[kN/m ³]	["]	[kN/m ²]	[m]		[kN/m ²]	[kN/m²]		[kN/m²]	[kN/m ²]		[kN/m ²	[kN/m²]		[m/s]		
Table	i -	-	-	17.00	10.00	32.5	0.00	2.00	0.20	6000.00	700.00	0.50	1000.00	3500.00	direct specification	✓ 4946.	2473.00	0.50	1E-07		0.003
															from constrained modulus						
Table	•				Def	ined	layers	/valu	es c	an be	selec	cted v	via a s	oil lay	er library.						
Cate	jory				Soil bea D of	cate ring (f NF-	gory a capac P94-2	accor ity ca 61.	ding Icula	to Ar ation f	nex A from v	A of s value	standa es of tl	rd NF ne pre	P94-261. It is i essiometer test	mpor accoi	ant for ding to	the Anne	∋x		
Nam	e				A na	ame	for th	e soil	laye	r can	be as	sign	ed her	e.							
Symb	ol				An a	abbre	eviatio	on for	the	soil la	iyer ca	an be	e assię	gned h	nere.						
хU					thic	knes	s of tl	ne so	il lay	er. So	il laye	ers si	maller	than	0.10 m are not	provi	led.				
V				The Poisson's ratio defines the ratio of a change in thickness to a change in length as soon as a stress is applied. The Poisson's ratio or transverse contraction coefficient has the formula v or μ . It is one of the elastic material constants and bears the name of the physicist Siméon Denis Poisson.																	
Em					Def sett	Define the pressiometric modulus according to Ménard here. It is needed for the settlement calculation from data of a pressiometer test.															
ΡI				The representative value of the limit pressure according to Ménard in the foundation base of the shallow foundation.																	
α					Rhe	olog	ical fa	ctor	for s	ettlen	nent c	alcu	lation	from	results of a pre	ssion	neter te	st.			
qc					The elas	peal sticity	k pres / and	sure frictio	resis on ar	stance igle fo	e com or bas	ies fr se fai	om th Iure a	e pres nd set	ssure test and c tlement calculation	derive: ation.	s modu	lus of	f		

Settlement analysis

Procedure	Direct specification or from constrained	Soil layer		?	\times					
	modulus:	Settlement Analysis								
	To define the compressibility of the soll (Em-	Procedure		direct specification						
	- directly in E* or - from the constrained modulus - Em will be	Compresses modulus	E*	[kN/m²]	4946.00					
		Stiffness module	Es	[kN/m²]	2473.00					
	calculated from stiffness/constrained modulus	Correction factor	х		0.50					
	Es and correction factor x (from DIN 4019 T1).	Settlement analysis: Consolidation								
E*	Compression modulus. The compressibility of	Permeability factor	k	[m/s]	1E-09					
_	the soil can be specified by a pressure	Both sides drained								
	settlement line or calculated from the constrained modulus in connection with a correction factor.									
Es	Stiffness/constrained modulus.									
х	Correction factor.									

Settlement analysis: Consolidation

ks	Permeability coefficient of the rate of consolidation. The value can be extracted from the soil report.
Both sides drained	For the calculation of the time to approximate decay of consolidation settlement in unilateral drainage the full layer thickness is set, in bilateral drainage only half the layer thickness.
Cα'	The creep coefficient Ca can be determined from a time-settlement test according to DIN 18135. Usual value range 0.001 to 0.00001.



0

1.00

0.00

+

0

1.00

0.00

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0

1.00

0.00

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Ground water

Ground water existing	This option allows you to define whether groundwater exists (displays the entry "Ground water").				
Ground water	Only if ticked option "Ground water existing". Absolute depth of the groundwater below the bottom edge of the foundation body. Negative values can be used to define a groundwater level below the base of the foundation.				
Surface					
Anchoring depth	Anchoring depth of the foundation body.	Properties		д	
Additional Terrain load	Additional characteristic permanent area load on the bearing failure figure, which increases the characteristic punching shear resistance.	Basic parameter System Foundation Column Soil	Q (2)		
Slope	The ground level can be modeled as horizontal, with a continuous slope, or with a broken embankment.	Ground water Surface ⊕ Loading			
	Continuous: Here you can define the slope and a berm -				
	see <u>extended soil mechanics dialog</u> .	General		0	
		All around the same			
	Broken:	Ground right (+X)			
	Input of the embankment sections. The "+" symbol	Anchoring depth	[m]	1.00	
	creates a new table row for a further section. Parameters	Additional terrain load	[kN/m ²]	0.00	
	automatically to the incline)	Slope	continuous	-	
	automatically to the melline)	Berm	none		
		Inclination B	broken		

Four-sided different terrain definition with the additional option FDPro

With an existing FDPro license, the terrain can be defined differently for each of the four foundation faces. To do this, remove the tick from the "All around the same" option - the entry will be extended accordingly.

Slope

Slope

Slope

Ground left (-X)

Anchoring depth

Ground top (+Y)

Anchoring depth

Additional terrain load

Additional terrain load

Ground below (-Y)

Additional terrain load

Anchoring depth

[m]

[kN/m²]

none

[m]

[kN/m²]

none

[m]

[kN/m²]

none



p

0 \checkmark

3 \checkmark

0 Z

Y base 90

Loading

Figongowicht "	Automatische Perücksichtigung des Eigengewichtes – Pei	Properties	
Eigengewicht γ	Grundwasser oberhalb der Sohle lässt sich das Eigengewicht nicht deaktivieren.	Basic parameter B. System Coading	
H loads base	 Option not ticked: The horizontal loads apply at the top edge of the base and generate a moment with a particular lever arm Option ticked: The horizontal loads apply directly in the base joint without generating a moment. 	… Load Cases … Single Loads … Line Loads … Area Loads ⊕ Design ⊕ Output	
		Loading	
Delete horizontal loads	Delete all horizontal loads with one click!	Self-weight	Y
	This is useful if many load cases from other applications	H loads apply	base
	(GEO, B5) have been imported.	Delete horizontal loads	
	Note: The horizontal loads of the individual load cases can	right-handed coordinate system	
	be found/entered under the following point "Load Cases".	Snow accidental	
Right-hand coordinate s	ystem (new standard)	Remarks	
5	Coordinate system based on the right-hand rule, also	to the effects	
Appidental approved	referred to as right-hand coordinate system. The signs comply with the sing definitions in engineering mechanics. P the x-axis generate pressure on the bottom and/or in the neg foundation. Positive moments about the y-axis generate press in the positive X-area of the foundation. If this option is unche until recently) positive moments generate pressure on top rig X/Y-area of the foundation. In the graphic representation, bot with their absolute values. The arrows indicate the actual dire values in the data entry fields and in the output documents an signs. If you change the sign definition, the sign of the mome changes as well.	ositive moments a ative area of the sure on the right a ecked (default set jht and/or in the p h variants are sho ection of action. T re indicated with t ents about the y-ax	about and/or ting ositive wn he heir kis
Accidental snow load	When you check this option, snow loads are automatically ind action in addition to the typical design situations. The user ca selectable load factor for the accidental snow loads or have i automatically by the software. The default value is 2.3	cluded as acciden an either specify a it determined	tal freely

Remarks

The remarks editor is called up via the 📝 button. This text appears in the output.



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90

Properties

Loading

⊕ Design ⊕ Output

Load Cases

Load Case

Column Loads

Basic parameter

- Single Loads - Line Loads

Area Loads

LoadCases

Enter the data of the first load case via the data-entry mask or directly in the

load case table, which can be displayed by activating the Load case tab (below the graphic).

Load case toolbar: Load case 0 1/2 0 👍 🗙 🗃 🗃 🍃 See Data entry via tables

To add additional load cases, click on the button once more (a new empty input mask is displayed each time).

Tip: A description is displayed in the status line each time you click into an input field.

Column Loads

		Description			Load case 1
Description	Optional text to the selected action can be entered.	Action		Permanent loads 🔹	
	This text is included in the output.	Vertical force in z	k	[kN]	500.0
Action	The appropriate actions can be selected from a list:	Moment about x	Th.1.0,k	[kNm]	0.00
	Permanent loads seismic loads	Moment about y	Th.1.0,k	[kNm]	0.00
	(calculation method "characteristic").	Horizontal force in 2	c Th.1.0,k	[kN]	0.0 🛄
Vertical force in z	Vertical force in the centre of the column	Horizontal force in y	/ Th.1.0,k	[kN]	0.0 🖬
Moment about x/y	Positive moments generate pressure on top right or	Group membershi	р		0
5	in the positive x/y section of the foundation.	Simultaneous group			0
Horizontal Force in x/v	Horizontal loads apply to the top edge of the	Alternative group			0
If "Design values" was s	are taken into account automatically by the software.	appear:			
Туре	G/G+Q. For consideration in the calculation of the gapi no gaping joint may occur, for permanent and variable reach the foundation centre point at the most. For perm theory only, the gaping joint is also tolerated up to the o	ng joint. For pern loads the gaping nanent loads fror centre of the four	nanent Ic joint ma m 2nd or ndation.	oads y der	
Limit state	STR - internal failure of components e.g. bending meas GEO - failure of the foundation soil e.g. bearing resistan EQU - Loss of stability. UPL - Verification against uplift or floating. SLS - Verification of serviceability e.g. settlement or ver The limit state together with the design situation provid the safety factors to be used. If there are no 4 load cass for the limit states STR/GEO-2, EQU, UPL and SLS, miss automatically generated with the help of the reduction construction verifications, the limit states SLS and STR simultaneously. Missing limit states are generated by of factors. SLS x reduction factor = STR/GEO-2 and STR/GEO-2 / r	surement. Ince failure. Ince failure. Infication of gapin des sufficient info es with the same sing load cases a factors. For som C/GEO-2 are requi offsetting with re-	ng joints ormation e designa ire e basic ired duction = SLS.	for ation	
Design situation	Selection of the design situation (permanent, temporar	y, exceptional, ea	arthquak	e).	

Reduction factors

These input fields are enabled if "Design values" was selected as calculation method.

Reduction Factor N Reduction coefficient for the forces acting in the z-direction (axial force in the column) and loads (additional concentrated loads, line loads and surface loads).



Reduct. Factor Others Reduction factors for other internal forces. If a column was designed in a second order analysis, the internal forces are only available on the design level. In order to make verifications in soil engineering available on the characteristic level, the reduction factors are used to adjust the internal design forces to a characteristic level. When using the characteristic calculation method (> Basic parameters > Calculation method) in combination with first-order columns, the afore-mentioned situation does not occur.

Group membership

The assignment to a group is displayed if "characteristic" has been selected under
Basic parameters Type of actions.

Simultaneous (concurrent) group

Loads of a particular action group can be defined as "always acting simultaneously" by assigning them to simultaneous (concurrent) groups.

III.: Example for the functioning of alternative and simultaneous groups



Alternative group

Different variable load cases with similar actions can be assigned to an alternative load case group via the allocation of an <u>alternative group number</u>. Only the decisive load case of this alternative load case group is invoked in the superposition.

Bearing pressure / Actions from the column

Display of the bearing pressure pattern

To ensure traceability, the bearing pressure pattern with stress can be shown for all load cases and superpositions decisive in the verifications. Click the symbol "Bearing pressure" to display the graphic in a popup window. See also → Design → Soil mechanics.



Nz.k



Single Loads

Define a new concentrated (single) load by activating the ^b button (the corresponding input mask is displayed).

Activating the Single load table" giving an overview of the defined loads.

Toolbar: 0 1/2 0 🍙 🛪 💷 🖉 - see also Data entry via tables

- Tip:A description is displayed in the status line each time you click into
a particular input field.
- In all LC: For "<u>Type of actions</u>" = design values: if the option is marked, the concentrated load acts in all load cases.
 - Value of the axial force of the additional single load. By clicking on the arrow icon vou can access a load value summary see the description of the LOAD+ application.



Single Loads			0
Single load	0 1/	1 0 👍	× 🗃 🖬 🌶
Nz	k	[kN]	0.0
at	ax	[m]	0.00
at	ay	[m]	0.00
Active in load ca	ise	1	2

at ax/ay Position of the additional single load in x or y direction referenced to the foundation centre.

Active in LC Assignment of the additional single load to load cases.

Activating the button is displays a dialog with the corresponding options.

Notes: If a single load is assigned to one or several load cases it acts only in combination with the load case(s.

In the case of the calculation method <u>design values</u> single loads are processed with the corresponding <u>reduction factors</u>.

Single loads that are not assigned to load cases are <u>not</u> taken into account in the calculation.

All verifications are referenced to the column loads. Additional single loads are defined only to check the effects on the bearing pressure, tilting, position stability, sliding and ground failure.

For the verification of punching shear resistance, the loads that apply in the area of the punching cone must be summarized to a resulting load, because the shear design would be unsafe otherwise.

With foundations for twin columns you should combine both columns to a single column instead of defining the second column as an additional single- or line load. Otherwise, you will obtain incorrect results in the verification of punching shear resistance.

Line Loads

General operation as described under single loads.

In all LC:	For "Type of actions" = design values: if the option is marked, the load acts in all load cases.
P1,k	Value at the begin of the line load. Alternatively, the type of action can be changed to 'Design values' in the <u>basic</u> <u>parameters</u> . If parts of the line load are located in examined circular sections of the punching shear check, these are taken into account when determining the shear stress. No extra punching shear check for a wall end or similar is carried out for the line loads. The punching shear check always refers to the column load.
at x1/y1	Position of P1 relative to the foundation center
P2,k	Value at the end of the line load
at x2/y2	Position of P2 relative to the foundation center
Active in load case	As described under single loads



Area Loads

In all LC:	For " <u>Type of actions</u> " = design values: if the option is marked, the load acts in all load cases.
Loads by soil on the fou	hdation Height of the earth surcharge, if applicable. In combination with the weight density γ, the soil load generates an area load on the foundation, which is taken into account in the calculation. Explanatory note: The earth surcharge load refers to the top edge of the foundation. If a wall, column, wall base or pocket exists, the earth surcharge load is reduced in accordance with the geometry of the structural component.
	Note: This value has nothing to do with the self-weight of the foundation.
Density γ,k	Weight density of a possible soil load.
Area Load q,k	Additional area load on the foundation body. Explanatory note: The area load acts on the surface of the foundation. If a wall, column, wall base or pocket exists, the area load is reduced in accordance with the geometry of the structural component. If a top-mounted pocket exists, the area load also acts on the pocket, but not in the area of a column casted in the pocket. See the description of the option "Earth surcharge height" for more information.
Active in load case	Assignment of the additional area load to load cases.
	Activating the button $ectical$ displays a dialog with the corresponding options.



Design / Verifications

Settings		Properties		Д
Earthquake: Psi ₂ =0.5	In accordance with the introductory decree of DIN 4149 for Baden-Württemberg, the combination coefficient Psi2 = 0.5 for snow loads should be used in the superpositions with seismic loads.	Basic parameter System Loading Design Soil Mechanics		20
Round out	This setting only affects the graphic representation of the course of the internal forces. It has no influence on the calculation results.	… Earth pressure … Parameter ⊕ Output		
Transient situation	When you check this option, the transient design	Settings		0
	situation is used. When you uncheck the option, the	Earthquake: Psi2=0,5.		
	persistent situation is used. The accidental	Round out the course of the in	ternal forces V	\checkmark
	situation and the seismic situation are	Round out the course of the in	ternal forces M	
	automatically considered if corresponding actions	Transient situation		
	have been defined.	Remarks		0
		to the results		1
Soil mechanics				
Proof format	Define here whether a	Properties		п
	- simplified verification, an	Properties Basic parameter		
	 exact verification or a user-defined verification is to be carried out. The <u>simplified verification</u> includes compliance with 	esic paraneter e - System e - Loading c - Design - Soit Mechanics - Earth pressure		Q ⊗
	the design value of the bearing resistance with limitation of the inclination of the load resultants. The exact verification format includes a foundation	⊕ Output		
	failure verification, a sliding safety verification and a	Allgemein		0
	settlement calculation.	Proof format	User defined	-
Checks soil engineering	Click on this symbol to open the <u>extended dialog</u> with graphical illustrations for bearing failure bearing	Checks soil engineering EQU - Stability	User defined simplified exact	
0	pressure and settlements.	Static equilibrium		
Bearing	You can also open the extended dialog directly in the	UPL - Uplift		0
Flessule	toolbar with the bearing pressure symbol (note: if only	Abheben		
	the simplified verification is carried out, only the	GEO - bearing capacity -	simplified verific	ations 🔕
	"Bearing pressure" tab is displayed).	Resulting bearing pressure		
		Bearing resistance		
User-defined proof format		Ausmittenbegrenzung		
All varification options are a	offered here for individual calentian	Nachweisumfang		
		GEO - bearing capacity -	precise verificati	on 🔕
Resulting bearing pressure	Requirement for the simplified verification: the	Proof of sliding capacity		
	Inclination of the characteristic or representative	Ground failure check		
	bearing pressure resultant complies with the	Seismic		
		Earthquake zone	2	
Bearing resistance	The verifications for the ground failure and sliding	Depth factor	without	
	infinit states as well as the serviceability (verification	SLS - Serviceability - pred	ise verifications	0
	values for the design value of the bearing resistance.	Calculate settlement	without	•

Limitation of eccentricityVerification according to NF P 94-261 13.3 for the
eccentricity of the load.Scope of verificationIn a separate dialogue, you define whether the limit

n In a separate dialogue, you define whether the limit states and design situations according to the

SLS - Serviceability - simplified verifications

Gaping joint

Nachweisumfang

Limit edge stresses

~

1



	selected standard are to be used for this verification or whether they adapted individually (user-defined).	are to be
Proof of sliding capacity	When the load vector is not perpendicular to the base surface, the rest the foundations against sliding in the base area must be verified.	sistance of
Seismic	With the add on FDPro: run a seismic bearing capacity detection according to the set of	ording to DIN
Depth factor	The depth coefficients take into account the favorable influence of th strength in the fracture joint above the base of the foundation in the base failure analysis. In some European countries, this effect can be taken account with coefficients > 1.	e shear bearing into
Calculate settlement	For the settlement analysis, the compression of the soil should be taken into account down to the settlement influence depth ts. The depth ts may be assumed at the level at which the additional perpendicular stress generated by the mean settlement effective load has an amount of 20% of the effective vertical output stress of the soil. One of 5 calculation methods can be selected.	without Settlement equations Stress integration from pressure meter test data from cone penetration data adapted elasticity procedure
Gaping joint	Optional verification of the gaping joint.	

Extended soil mechanics dialog



Calling up the dialogue on "checks soil engineering		Soil Mechanics				
		Ground failure Settlemen	nt Diagram	s subsidence Be	aring Pre	ssure
		Ground failure				0
Ground failure		Check bearing resistance				
		Seismic				
Seismic/Earthquake zone	: call up the earthquake dialog.	Earthquake zone	2	2		1
Selection of the partial sa	fety factor γRd.	Teilsicherheitsbeiwert yRd	=1.15 Loosel	y stored dry sand		•
	5	Surface				0
0		Surface				
Surface		Ground water				0
The following input param	neters are displayed via the	Ground water existing			1	
"Surface" button:		Ground water	[m]		1	1.00 🜲
Anchoring depth	Lowest anchoring depth below terrain/ top of basement sole.	Terrain			?	×
<u>Slope</u>	The ground level can be	Anchoring depth	Anchoring depth			0.50
	horizontal, with a <u>continuous</u>	Base		continuous		0.00
	slope, or with a <u>broken</u>	Inclination	ß	[11]		10.0
	<u>embankment</u> .	Additional terrain load	P	[kN/m²]		0.00
Berm	The width of berm is the distance between the outer edge slope.	of the foundation and	the beginn	ing of the		0.00
Inclination β	The terrain inclination indicates t defined berm. The inclination affe exclusively downsloping terrain.	he angle of inclination e ects the ground failure	of a slope verification	from the n and defines		
Additional terrain load	An additional characteristic perm can be entered here, which increat resistance.	nanent area load on the ases the characteristic	bearing re punching s	esistance figure shear	Ĵ	
Ground water						

Groundwater exists	See System ▶ <u>Groundwater</u> .
Groundwater Depth	See System ▶ <u>Groundwater</u> .

Settlement

Representation of the course of settlement and stress over the depth as well as graphic representation (diagrams of settlement) of the course of settlement over time, the settlement and influence coefficients per selection list.

Calculate settlement	
Settlement Gk,j	

see page before

Settlements can be calculated with permanent loads or with permanent and variable loads.

Soil Mechanics Ground failure Settlement Diagrams subsidence Bearing Pressure Settlement

Calculate settlement		without	
Settlement Creep settlements		without	
		Stress integration	
Time τ Earth pressure	rom pressure meter test data		
		adapted elasticity procedure	
Use earth pressure			\checkmark
Groundwater			0
Ground water existing			

0



Bearing pressure

Display of the bearing pressure pattern in 2D/3D. Selection via the top selection line.

For entries/changes, see the chapter System > Soil.

Bearing Pressure

Simplified check Simplified check Gaping joint permanent loads only Gaping joint permanent and variable loads Ground failure

Earth pressure (with the add-on FDPro)

Enables the application of earth pressure. See FDPro



Passive earth pressure enabled	
	0
Earth pressure type	Active earth 🔹
Increased active earth pressure	
apply tensile forces from cohesion	
Apply minimum earth pressure	\checkmark
Apply compaction pressure	



Parameter

User defined

Mark this option if you want to change the safety factors and design rules that deviate from the set standards.

The corresponding input fields/editing buttons are then displayed.

Use the "Edit" button to open the respective tables for changing the values - the information texts for the individual parameters are displayed in the lower window area when you click in an input field.

- Support of all 3 verification methods according to Eurocode 7, adjustable for all national annexes.
- The partial safety factors and combination equations for the geotechnical verifications can be edited.
- Since all table values can be changed, the standard setting for a specific country (e.g. India, Sweden, etc.) can be easily defined.

Properties	4
Basic parameter	0 🚳
	20
E. Design	
Soil Mechanics	
Earth pressure	
Parameter	
. Output	

General Settings		0
User defined		
User defined values	->	Edit
User defined values	->	Default valu
All safety factors		Edit (53)
Combination equations		0
Verification procedure	1	Edit (2)
Verification procedure	2	Edit (2)
Verification procedure	3	Edit (2)
Failure of structures and	d componer	nts 🔕
Action/Strain	STR A	Edit (4)
Material resistance	STR M	Edit (2)
Failure of subsoil		0
Action/Strain	GEO A	Edit (10)
Material resistance	GEO M	Edit (10)
Lad resistance	GEO R	Edit (6)
Stability		0
Action/Strain	EQU A	Edit (4)
Material resistance	EQU M	Edit (5)
Float up		0
Action/Strain	UPL A	Edit (4)
Material resistance	UPL M	Edit (5)



Output

Output scope / options

By checking the desired options, you can determine the scope of output. Font size and scale can be adjusted for the graphic.

Output as PDF document

The Document tab displays the document in PDF. See also <u>Output and printing</u>.

Properties	д
Basic parameter	0 @
	20
⊕. Design	
- Output	
General	
Soil Mechanics	

	0			
User defined	•			
	0			
Static equilibrium				
UPL - Uplift				
ed verifications	0			
Resulting bearing pressure				
Bearing resistance				
ise verification	0			
verifications	0			
	\checkmark			
	\checkmark			
ed verifications	0			
	User defined ed verifications ise verification verifications ed verifications			

lan view	<u>Loads</u> Column le	<u>Loads</u> Column loads - characteristic									
olumn loads	No.	Act	Description		N kN	Mx kNm	My kNm	Ha kN	Hy kN	SIM	ALT
Superposition Results Preview Checks Preview Checks Simplified method	1 2	g A	Load case 1 Load case 2		500.0 300.0	0.00	0.00	0.0	0.0	0	0
	Self-weigh resp. colu <u>Superpo</u>	Self-weight is taken into account in the calculation. Density Concrete : y = 25.00 kN/m ³ . Total Foundation without socket resp. column 20.000 m ³ / 500.00 kN. Torsion from horizontal loads is not considered. Superposition									
1	N	o.	DS	Superposit	ion						
		1 P 2 P 3 P 4 P 5 P 6 P		0.9 resp. 1.1x(1) 0.95 resp. 1.05x(1) 1.0x(1) 1.0x(1) + 1.0x(2) 1.35x(1) + 1.5x(2) 1.0x(1)							
	DS: de sig The load	DS: design situation P: Permanent The load case numbers are listed in parentheses.									
	Results Preview C	hecks									
	Check	Check						Superposition ŋ			η
- - - - - -	Gaping jo Gaping jo Stability Simplified	Gaping joint only permanent bads 3 0. Gaping joint permanent and variable loads 4 0. Stability 1 0. Simplified method 5 0.							0.00 0.00 0.00 0.26		
	Design va ord = 350.	Design value of the bearing pressure resistance $\sigma_{R,d} = 350.00 \text{ kN/m}^2$ $\sigma_{Rd} = 350.00 \text{ kN/m}^2$. The design value of bearing pressure resistance has been specified directly.									
2	Simplified	metho	a Superpositio	'n						-	
	No.		Nd kN	a' m	b' m	kN	σd I/m²	kN	ord V/m²		n
	5	_	1800.0	4.00	5.00	9	0.00	35	0.00		0.26